# Memorandum 

To: Rick Goncalves, TAC Chairman<br>Cc: Chuck Reid, Manager<br>From: William P. Ruzzo, P.E., Craig Wolf, GEI

Date: January 22, 2014, Amended February 27, 2014
Re: Cherry Creek Reservoir Area-Capacity Data, Amended

The Cherry Creek Basin Water Quality Authority (Authority) has retained Hydros Consulting Inc ${ }^{1}$ to develop a reservoir water quality model. Important input data for the model includes the reservoir surface area at regular intervals below the reservoir surface. To develop this data, the Authority also retained Absolute Natural Resources, LLC $^{2}$ (ANR) to prepare a bathymetric survey, which determines the depth from the water surface to the reservoir bottom.

ANR conducted the survey in November 2013, processed the data to develop one foot depth contours, and provided the Authority an AutoCAD file with the depth contours in the reservoir. This file was then used by the Authority to calculate surface area and storage volume of the reservoir. The methodology and approach used to prepare the area and volume calculations are described herein and the results presented in tabular and graphic formats.

Based on the ANR 2013 survey, the volume of the reservoir below the normal pool elevations (multipurpose storage pool) was determined to be 13,522 acre feet (ac ft) and presented in Local Project Datum. The surface area at the multipurpose storage pool elevation of 5,550 feet (ft) was determined to be 875 acres (ac).

This memorandum, which replaces the previous memorandum dated January 22, 2014, was amended to include an investigation to determine the appropriate vertical datum for the bathymetric survey and to further compare area and storage volume results to values recently published by the U.S. Army Corps of Engineers (USACE July 2011). The amendment was necessary because it is important for the bathymetric results to be consistent with the elevation information reported by the USACE on their website which is also used by the Colorado Division of Water Resources to manage water rights in Cherry Creek. In addition, the

[^0]multipurpose storage pool elevation of $5,550 \mathrm{ft}$ is used by the Authority to establish protection elevations for shoreline stabilization projects, such as the Mountain and Lake Loop project completed in 2013.

## METHODOLOGY

ANR conducted a sub-foot mapping survey using a professional grade GPS (differential and RTK) hydro-acoustic (sonar) and GIS technology to collect 298,925 data points (i.e.: $\mathrm{x}, \mathrm{y}$, and z coordinates). The survey was performed between November 18 and November 27, 2013 by mounting the instrument on a small motorized boat. These spatial data points were used to develop waterdepth contours at one-foot intervals and create a contour map of the reservoir using proprietary software (Figure 1 - Cherry Creek Reservoir). ANR provided multiple files to the Authority
 that included AutoCAD and GIS shape files, laminated maps, and a Google Earth® file with reservoir water depth contours at 1 -foot intervals beginning with the "zero" depth at the normal pool elevation of $5,550 \mathrm{ft}$ (Local Project Datum).

The AutoCad file was modified by the Authority to facilitate calculation of surface area at each water depth as follows:

1. Each 1-foot depth contour (39-each) was placed in a separate CAD layer.
2. The contour information, which typically consisted of multiple data blocks, was then "exploded" to create individual polylines. Because there were often multiple individual contour lines for each elevation, depth specific contour lines were "chained" together, where possible, to create one continuous polyline for each depth contour. Contour lines in the area of the outlet tower were found to stop at the outlet tower limits. To facilitate area calculation, the contours surrounding the outlet tower were connected to create complete polylines.
3. The area of each depth specific contour was then calculated using a CAD routine and recorded in an Excel spreadsheet. The volume between each depth layer was then calculated using the equation:

$$
V=\frac{D}{3} *\left(A 1+(A 1 * A 2)^{0.5}+A 2\right)
$$

Where $\quad \mathrm{V}=$ Volume
$\mathrm{D}=$ Depth between contours (1-foot)
A1, A2 = Area at upper and lower contour.
Calculations for cumulative volume were then added to the Excel spreadsheet to determine the volume of the multipurpose pool below $5,550 \mathrm{ft}$.

## DATUM DIFFERENCES

During the Authority's review of the results published in the original memorandum, it was discovered that the stated vertical datum on the work products was not consistent, with notations contained in electronic files that the datum used was the NGVD29 or NAVD88 datum. The Authority recalled the original memorandum and began investigating the datum discrepancy, the findings which are discussed below.

ANR based their survey on elevations reported on the NOAA website
(http://water.weather.gov/ahps2/hydrograph.php?wfo=bou\&gage=egdc2) and confirmed that the NOAA data was obtained from the USACE gaging site at the Reservoir ${ }^{3}$ by contacting personnel at NOAA.

Elevations reported by the USACE on their website are based on Local Project Datum, not NGVD29 ${ }^{4}$ or NAVD88 datum. The Local Project Datum, used by the USACE in reports for Cherry Creek, is approximately $1.27 \mathrm{ft}^{5}$ above elevations based on NGVD29 datum. The relationship between NGVD29, Local Project, and NAVD88 datum is provided in the Appendix 2. Whereas the datum differences are shown to a precision of 0.01 ft , discussions with the USACE suggest that the precision is overstated and has varied over time.

For the Mountain and Lake Loop Shoreline Stabilization project, which first discovered the datum differences ${ }^{6}$, NGVD29 elevations were found to be 0.97 ft lower than elevations shown on the USACE website ${ }^{7}$. The difference of 0.97 ft does not agree with the reported USACE difference of 1.27 ft . The 2011 USACE report does discuss that the staff gage has settled $\sim 0.2-$ ft but they have confirmed via personal communication that the staff gage is not used to monitor pool elevation, so this offset is not the basis for the discrepancy between the two elevation comparisons either.

## REASONABLENESS CHECK

To check the reasonableness of the area and multipurpose pool volume calculations, as based on the ANR analysis, results were compared to recent data published by the USACE and independent calculations by Leonard Rice Engineers (LRE). A comparison of all three sources is presented in the table below:

| Data Source | Surface Area | Storage Volume |
| :--- | :---: | :---: |
|  | (acres) | (acre feet) |
| ANR 2013 | 876 | 13,522 |
| USACE 2011 | 840 | 12,558 |
| LRE 2013 | $\mathrm{n} / \mathrm{a}$ | 13,504 |

[^1]USACE Tri-Lakes Sediment Report. The USACE performed cross-section surveys collecting elevation data in 2006, 2007, and 2009 to determine the change in reservoir volume resulting from sedimentation. The results are presented in their 2011 report in which elevation data are reported based on Local Project Datum. The USACE compared cross section data to the original project elevations to determine changes in area, and therefore volume, at each cross section. A copy of the USACE results in tabular form for surface area and for capacity at onefoot intervals is provided in the Appendix 2 to this memorandum and is reproduced in Table 2. The USACE calculated the volume to be 12,558 ac ft , a difference of 964 ac ft or $7.1 \%$ when compared to results based on the ANR survey. Similarly, the surface area was calculated by the USACE to be 840 ac , a difference of 36 ac or $4.1 \%$.

LRE Calculations. A copy of the modified ANR GIS shape files were provided to LRE who performed an independent calculation of the reservoir volume (multipurpose storage pool) using GIS software rather than computational formulas in Excel. LRE calculated the volume to be $13,504 \mathrm{ac} \mathrm{ft}$, a difference of 18 ac ft or $0.13 \%$ when compared to Excel results based on the ANR survey. LRE calculations are provided in Appendix 1 to this memorandum.

## RESULTS

Results of the survey and analysis are presented as tables and figures attached to this memorandum and discussed below. All elevation information presented in these results are based on the Local Project Datum (Local Project Datum $=$ NGVD29 +1.27 feet).

Table 1 presents the results of the 2013 ANR survey and subsequent calculations by the Authority of the surface area and accumulative volume at each contour (i.e.: from elevation $5,512 \mathrm{ft}$ to elevation $5,550 \mathrm{ft}$, the multi-purpose storage pool ${ }^{8}$ ). Figure 2 is a plot of the storage area as a function of elevation (i.e., stage) and Figure 3 is a plot of the surface area at each contour. Between elevation $5,512 \mathrm{ft}$ and $5,523 \mathrm{ft}$, the area and volume are very small compared to the area and volume at the multipurpose storage pool elevation of $5,550 \mathrm{ft}$. This is due to sediment accumulation at the downstream end near the Outlet Tower which is frequently flushed out by the USACE and resulted in a deep hole at the outlet structure.

Table 2 provides a comparison between the Authority's calculations based on the ANR survey and the USACE calculations for each elevation ${ }^{9}$. Figure 4 is a plot comparing the storage volumes and Figure 5 is a plot of the surface area for both the ANR and the USACE surveys. As shown in Table 2, the surface area calculations differ from as little as 1 ac up to 99 ac. The storage volume at each elevation differs from 1 ac ft up to 964 ac ft at the normal pool ${ }^{10}$.

## FINDINGS AND CONCLUSIONS

The USACE determined the storage volume at $5,550 \mathrm{ft}$ to be $12,558 \mathrm{ac} \mathrm{ft}$, which is $964-\mathrm{ac} \mathrm{ft}$ (7\%) less than the 2013 ANR survey ( $13,522 \mathrm{ac} \mathrm{ft}$ ). The USACE surface area at elevation 5,550

[^2]is 840 ac which is 36 ac ( $4 \%$ ) less than the 2013 ANR survey ( 876 ac ). Possible explanations for these differences are discussed below.

The storage volume derived from the 2013 ANR survey was based on the average end-area method using horizontal sections (i.e.: contours) at 1 -foot intervals. The storage volume calculated by the USACE used the average end-area method based on vertical cross sections that were hundreds of feet apart. The closer transect spacing used by ANR (e.g. 50 ft ) to achieve their sub-foot measurements is believed to result in a more accurate estimate of the storage volume.

According to the USACE sediment report, the accumulation of sediment in the reservoir has decreased from a peak of $161 \mathrm{af} / \mathrm{yr}$ in 1965 to $\sim 12 \mathrm{af} / \mathrm{yr}$ in 2009 (p.5-4). Therefore, additional sediment accumulation accounts for less than $5 \%^{11}$ of the difference and is not believed to be the reason for the difference in storage volume.

The USACE also used an alternate approach to surveying using LiDAR techniques ${ }^{12}$ that resulted in a storage volume of 13,926 ac ft at $5,550 \mathrm{ft}$, which is $3 \%$ greater than the 2013 ANR survey. Because the storage volume based on the 2013 ANR survey lies between the two estimates calculated by the USACE, the 2013 ANR survey results are believed to be consistent with USACE calculations.

Using GIS routines, LRE independently calculated the storage volume at the multipurpose pool elevation to be $13,504 \mathrm{ac} \mathrm{ft}$, which is less than $0.5 \%$ different from the average end-area methodology presented in this memorandum. Therefore, we believe the Authority's volume calculations are consistent with other estimates.

## RECOMMENDATIONS

Based on the investigations and findings described in this memorandum, the following recommendations are presented for the bathymetric results and future shoreline stabilization projects.

1. The Authority's depth specific surface area and volume calculations, based on the 2013 ANR survey (Table 1) are recommended for input to the Reservoir model. Using the Local Project Datum will allow correlation between the reported water surface elevations on the USACE, NOAA, or DWR websites and the model results.
2. When preparing design plans for PRF's in the Park, the USACE requires the work to be done in either NGVD29 or NAVD88 datum and to include an equation comparing the two. Because management of the storage levels in the reservoir are based on the Local Project Datum, it is recommended the Authority use the Local Project Datum and elevation $5,550 \mathrm{ft}$ to identify vertical stabilization limits for shoreline stabilization projects, which can be easily checked daily using the available websites. The 2011 USACE report shows that the annual maximum and minimum water surface elevation in the reservoir typically vary around elevation 5,550 ft Local Project Datum ${ }^{13}$.
[^3]
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## CHERRY CREEK BASIN WATER QUALITY AUTHORITY

Table 1 - Reservoir Volume and Surface Area

Normal Pool El $=5550.0$

| Depth | Contour Elevation | Area (acres) | Volume <br> Acre Feet |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | Vol. | Cumulative |
| -38 | 5512 | 0.0 | n/a | 0 |
| -37 | 5513 | 0.0 | 0.0 | 0 |
| -36 | 5514 | 0.0 | 0.0 | 0 |
| -35 | 5515 | 0.0 | 0.0 | 0 |
| -34 | 5516 | 0.0 | 0.0 | 0 |
| -33 | 5517 | 0.0 | 0.0 | 0 |
| -32 | 5518 | 0.0 | 0.0 | 0 |
| -31 | 5519 | 0.0 | 0.0 | 0 |
| -30 | 5520 | 0.1 | 0.1 | 0 |
| -29 | 5521 | 0.1 | 0.1 | 0 |
| -28 | 5522 | 0.2 | 0.1 | 0 |
| -27 | 5523 | 0.5 | 0.4 | 1 |
| -26 | 5524 | 1.9 | 1.1 | 2 |
| -25 | 5525 | 25.8 | 11.6 | 13 |
| -24 | 5526 | 165.1 | 85.4 | 99 |
| -23 | 5527 | 239.7 | 201.3 | 300 |
| -22 | 5528 | 279.8 | 259.5 | 560 |
| -21 | 5529 | 315.0 | 297.2 | 857 |
| -20 | 5530 | 344.7 | 329.7 | 1187 |
| -19 | 5531 | 387.3 | 365.8 | 1552 |
| -18 | 5532 | 416.6 | 401.9 | 1954 |
| -17 | 5533 | 440.2 | 428.3 | 2383 |
| -16 | 5534 | 467.6 | 453.9 | 2836 |
| -15 | 5535 | 490.6 | 479.1 | 3315 |
| -14 | 5536 | 514.5 | 502.5 | 3818 |
| -13 | 5537 | 546.2 | 530.3 | 4348 |
| -12 | 5538 | 563.9 | 555.0 | 4903 |
| -11 | 5539 | 590.2 | 577.0 | 5480 |
| -10 | 5540 | 613.8 | 602.0 | 6082 |
| -9 | 5541 | 635.1 | 624.4 | 6707 |
| -8 | 5542 | 656.8 | 645.9 | 7353 |
| -7 | 5543 | 683.1 | 669.9 | 8022 |
| -6 | 5544 | 711.6 | 697.3 | 8720 |
| -5 | 5545 | 742.0 | 726.7 | 9446 |
| -4 | 5546 | 772.2 | 757.1 | 10204 |
| -3 | 5547 | 804.2 | 788.1 | 10992 |
| -2 | 5548 | 833.5 | 818.8 | 11810 |
| -1 | 5549 | 857.0 | 845.2 | 12656 |
| 0 | 5550 | 875.5 | 866.2 | 13522 |

NOTE: Based on survey by Absolute Natural Resources performed November 18-27, 2013 Elevations are based on Local Project Datum. NGVD29 = Project Datum - 1.27'

## CHERRY CREEK BASIN WATER QUALITY AUTHORITY <br> Table 2 - Reservoir Volume and Surface Area - Comparison w/USACE

| Normal Pool El = |  | 5550.0 |  |  | USACE 2011 Appendix D |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Differences USACE - CCBWQA |
| Depth | Contour Elevation | Area (acres) | Volume <br> Acre Feet |  |  |  |  | Contour Elevation | Area (acres) |  | Area (acres) | Volume (acre feet) |
|  |  |  | Vol. | Cumulative | Cumulative |  |  |  |  |
| -38 | 5512 | 0.0 | n/a |  | 5512 |  |  |  |  |  |  |
| -37 | 5513 | 0.0 | 0.0 |  | 5513 |  |  |  |  |  |  |
| -36 | 5514 | 0.0 | 0.0 |  | 5514 |  |  |  |  |  |  |
| -35 | 5515 | 0.0 | 0.0 |  | 5515 |  |  |  |  |  |  |
| -34 | 5516 | 0.0 | 0.0 |  | 5516 |  |  |  |  |  |  |
| -33 | 5517 | 0.0 | 0.0 |  | 5517 |  |  |  |  |  |  |
| -32 | 5518 | 0.0 | 0.0 |  | 5518 |  |  |  |  |  |  |
| -31 | 5519 | 0.0 | 0.0 |  | 5519 |  |  |  |  |  |  |
| -30 | 5520 | 0.1 | 0.1 |  | 5520 | 0 |  |  |  |  |  |
| -29 | 5521 | 0.1 | 0.1 |  | 5521 | 0 |  |  |  |  |  |
| -28 | 5522 | 0.2 | 0.1 |  | 5522 | 0 |  |  |  |  |  |
| -27 | 5523 | 0.5 | 0.4 | 0 | 5523 | 0 |  |  |  |  |  |
| -26 | 5524 | 1.9 | 1.1 | 1 | 5524 | 0 |  | -2 | -1 |  |  |
| -25 | 5525 | 25.8 | 11.6 | 13 | 5525 | 61 | 17 | 35 | 4 |  |  |
| -24 | 5526 | 165.1 | 85.4 | 98 | 5526 | 120 | 122 | -45 | 24 |  |  |
| -23 | 5527 | 239.7 | 201.3 | 300 | 5527 | 152 | 258 | -88 | -42 |  |  |
| -22 | 5528 | 279.8 | 259.5 | 559 | 5528 | 184 | 426 | -96 | -133 |  |  |
| -21 | 5529 | 315.0 | 297.2 | 856 | 5529 | 216 | 626 | -99 | -230 |  |  |
| -20 | 5530 | 344.7 | 329.7 | 1186 | 5530 | 263 | 858 | -82 | -328 |  |  |
| -19 | 5531 | 387.3 | 365.8 | 1552 | 5531 | 321 | 1153 | -66 | -399 |  |  |
| -18 | 5532 | 416.6 | 401.9 | 1954 | 5532 | 369 | 1500 | -48 | -454 |  |  |
| -17 | 5533 | 440.2 | 428.3 | 2382 | 5533 | 413 | 1892 | -27 | -490 |  |  |
| -16 | 5534 | 467.6 | 453.9 | 2836 | 5534 | 452 | 2326 | -16 | -510 |  |  |
| -15 | 5535 | 490.6 | 479.1 | 3315 | 5535 | 486 | 2796 | -5 | -519 |  |  |
| -14 | 5536 | 514.5 | 502.5 | 3818 | 5536 | 515 | 3298 | 1 | -520 |  |  |
| -13 | 5537 | 546.2 | 530.3 | 4348 | 5537 | 540 | 3827 | -6 | -521 |  |  |
| -12 | 5538 | 563.9 | 555.0 | 4903 | 5538 | 559 | 4378 | -5 | -525 |  |  |
| -11 | 5539 | 590.2 | 577.0 | 5480 | 5539 | 574 | 4946 | -16 | -534 |  |  |
| -10 | 5540 | 613.8 | 602.0 | 6082 | 5540 | 587 | 5526 | -27 | -556 |  |  |
| -9 | 5541 | 635.1 | 624.4 | 6706 | 5541 | 605 | 6121 | -30 | -585 |  |  |
| -8 | 5542 | 656.8 | 645.9 | 7352 | 5542 | 627 | 6737 | -30 | -615 |  |  |
| -7 | 5543 | 683.1 | 669.9 | 8022 | 5543 | 649 | 7375 | -34 | -647 |  |  |
| -6 | 5544 | 711.6 | 697.3 | 8719 | 5544 | 673 | 8036 | -39 | -683 |  |  |
| -5 | 5545 | 742.0 | 726.7 | 9446 | 5545 | 698 | 8721 | -44 | -725 |  |  |
| -4 | 5546 | 772.2 | 757.1 | 10203 | 5546 | 725 | 9433 | -47 | -770 |  |  |
| -3 | 5547 | 804.2 | 788.1 | 10991 | 5547 | 752 | 10171 | -52 | -820 |  |  |
| -2 | 5548 | 833.5 | 818.8 | 11810 | 5548 | 780 | 10937 | -53 | -873 |  |  |
| -1 | 5549 | 857.0 | 845.2 | 12655 | 5549 | 810 | 11732 | -47 | -923 |  |  |
| 0 | 5550 | 875.5 | 866.2 | 13522 | 5550 | 840 | 12558 | -36 | -964 |  |  |

NOTE: Based on survey by Absolute Natural Resources performed November 18-27, 2013 All elevations are based on Local Project Datum. NGVD29 = Project Datum - 1.27'


Cherry Creek Reservoir
Arapahoe County, Colorado

Figure 2 - Stage Storage Curve


Figure 3 - Stage - Surface Area Curve


Figure 4 - Storage Volume Comparison ANR 2013 Survey and USACE 2009 Survey


## Figure 5 - Surface Area Comparison ANR 2013 Survey and USACE 2009 Survey



## APPENDIX 1

# LRE Cherry Creek Reservoir Volume Calculations Using GIS Techniques 

## Cherry Creek Reservoir Volume Calculation Using GIS Techniques

The following describes how the total volume of water in the Cherry Creek Reservoir was measured using GIS software, and the Cut/Fill tool in ArcGIS 10 starting with a bathymetric contour shapefile. The cut/fill tool measures the amount of gain, or loss between to elevation grids, to measure changes in volume or elevation, thus two raster grids were needed to run the tool: the first, derived from interpolation of the contour shape file showing depth of the reservoir in 2 ft . cell sizes ( $5514 \mathrm{ft}-$ 5549 ft ), and another mask grid, containing only the constant surface elevation of the water ( 5550 ft ) at equal 2 ft . cell sizes. After the two grids are obtained, the cut fill tool can be used to create a new grid showing and measuring the changes between the two elevation surfaces. The output measurement is shown by default as cubic units ( ft ), and then was recalculated in acre feet. The result of this calculation is $13,504 \mathrm{AF}$.


This represents a reasonable level of accuracy; however there are some sources where error could have occurred:

1) Repairing broken contours (as shown in following figures)
2) Interpolation of the depth Grid
3) Human error
4) Data accuracy

## References:

Price, ESRI, Mike. "Deriving Volumes With ArcGIS Spatial Analyst." ArcUser (2002) Web, Jan 2014

Original Contours:


## Reservoir Depth Grid:



Constant Surface Grid at 5,550 feet:


Cut Fill Results: 13,504 AF


Sources of Error? Broken Contours:


## APPENDIX 2

Email communications and extracted pages from USACE Tri-Lakes Sedimentation Studies AreaCapacity Report Revised July 2011.

## Cherry Creek Elevations

From : wfriesen@anrwildlife.com
Tue, Jan 28, 2014 04:43 PM
Subject : Cherry Creek Elevations
@1 attachment
To : bill ruzzo <bill. ruzzo@comcast.net>

## Bill,

We attained our elevation datum readings from the NOAA website at http://water. weather.gov/ahps2/hydrograph.php? wfo=bou\&gage=egdc2. Our data was justified to elevation readings daily at 12:00 noon. All readings were confirmed with NOAA's hydrologist Treste Huse (email : treste.huse@NOAA. gov) and comes directly from the Army Corp of Engineers gauging station at the reservoir. Please let me know if this is enough information.

Thanks,
Wesley Friesen
Project Biologist / Assistant Project Manager

## Absolute Natural Resources, LLC

5765 Olde Wadsworth Blvd., Ste. 10
Arvada, CO 80002
Voice: (720) 974-4075
Fax: (720) 974-4830

sigimg1
26 KB

From:
Sent:
To:

Subject:
Attachments:

Wolf, Craig [CWolf@geiconsultants.com](mailto:CWolf@geiconsultants.com)
Thursday, January 30, 2014 3:07 PM
Jean Marie Boyer; hawley@hydrosconsulting.com; Taylor Adams; Bill Ruzzo; Christopher Lewis ; Katie Fendel; Max Grimes (maxmgrimes@live.com); Myrna Poticha; Reid, Chuck; Rick Goncalves; Wolf, Craig
action items from conf call
CorpDatumDifferenceExplanation.pdf

Hello everyone,
I spoke with Ben Letak and Katie Seefus from the USACE and they report that USACE elevation data is based on their "local project datum" and NOT NAVD88 or NGVD29. The attached file summarizes the relationship between the three approaches, but Ben reaffirmed that the differences are not exact and he uses 1.8 and 1.3 for the differences. Katie also mentioned that the staff gage is not used to monitor pool elevation so the 0.2 offset reference in the memo does not apply to CCR elevations.

Because ANR adjusted their elevations to the USACE reported values and the 5,550.0 pool elevation, the ANR data should be in local project datum too....

The other time series flow and WQ data for 2013 calendar year were also transferred to Hydros so both action items have been addressed.

Please contact me if you have any questions.
Cheers!
Craig

## Craig F. Wolf

Senior Aquatic Ecologist / Limnologist
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Denver, CO 80237
Direct: 303.264.1028
Main Office: 303.662.0100
cwolf@geiconsultants.com
North American Lake Management Society: Region 8 Director
Colorado Lake \& Reservoir Management Association: Past President

### 2.4.2.3 Cherry Creek

The 2009 survey data presented in this report is a composite of data collected in 2006, 2007, and 2009. Cross sections CC-01 through CC-06 were completed in 2006 by in-house personnel. A contractor surveyed cross sections CC-07 through CC-11 and CC-13 in 2007. The hydrographic surveys completed during the 2006 survey were determined to be inaccurate and these sections, CC-01 to CC-04, were resurveyed in 2009 by in-house personnel. Note the CC-12 cross section was completely destroyed; survey data from 1988 was used for this cross section to run the area-capacity program for the latest survey.

## Brief Metadata: Cherry Creek

Survey Date: Data collected in 2006 (In-House), 2007 (A-E Contract), and 2009 (In-House)
Surveyor: In-House: USACE Omaha District, Hydrologic Engineering Branch,
Sedimentation \& Channel Stabilization Section
Contract Work: Ayres \& Associates, Inc., Fort Collins, Colorado
Horizontal Datum: Colorado State-Plane Coordinate System NAD83, Zone 0502.
Vertical Datum: Survey data was collected in the Local Proiect Datum.
Units: U.S. Survey Feet
Accuracy: $\quad 3^{\text {rd }}$ Order Horizontal \& Vertical per EM 1110-2-1003

A detailed Cherry Creek basin survey was performed using LiDAR mapping, flown on 12 February 2009 by Woolpert, Inc., Englewood, Colorado. The LiDAR data was provided to the Sedimentation and Channel Stabilization Section for comparison to the range line surveys. The LiDAR dataset was collected using vertical datum NAVD88.

The Cherry Creek Dam was built using elevations in a Local Project Vertical Datum. Based on a September 2010 survey, conducted by the USACE Omaha District Surveys \& Mapping Section, the Local Project Datum is approximately 1.27 feet above the NGVD29 datum and 1.76 feet below the NAVD88 datum. The NAVD88 datum is approximately 3.02 feet above the NGVD29 datum at Cherry Creek Dam, see Figure 2-2. Table 2-3 compares range line monument elevations as surveyed using the three datums. Any conversions between datums should be coordinated through the USACE Omaha District Surveys Section to ensure accuracy, these values are approximations. Elevations in this report for Cherry Creek are reported in the Local Project Datum.

The elevations on the Cherry Creek staff gage, which is located on the downstream side of the intake structure, are in Local Project Datum plus approximately 0.20 feet ( 1.47 feet above NGVD29 datum and 1.56 feet below NAVD88 datum). This 0.20 feet difference corresponds closely to the settlement observed in the upstream end of the conduits at the intake since original construction. If the staff gage is used to monitor pool elevation, subtract 0.20 feet from the elevation to approximate the elevation in Local Project Datum.


Figure 2-2. The Local Project Datum for Cherry Creek Relative to NAVD88 and NGVD29

## Bill Ruzzo

| From: | Melanie D. Chenard [mchenard@MULLERENG.COM] |
| :--- | :--- |
| Sent: | Wednesday, June 30, 2010 9:03 AM |
| To: | Rios, Alfredo A NWO |
| Cc: | Johnson, Daniel B NWO; Jim T. Wulliman; bill.ruzzo@comcast.net |
| Subject: | RE: Survey benchmark issues for shoreline project @ Cherry Creek Reservoir |

Fred,
Thank you for getting back to us so quickly. We will proceed with the design as you have indicated. The answer to item \# 2 does potentially impact our design as we look at the expected range of reservoir pool elevations - we need to make sure that we extend our shoreline protection to cover this range, which probably means protecting one foot lower than we had expected to cover the datum difference. We can proceed without an answer, but if you get one, wed appreciate hearing it.

Thanks again,
Melanie D. Chenard, P.E.
Muller Engineering Company, Inc.
303-988-4939
mchenard@mullereng.com
-----Original Message-----
From: Rios, Alfredo A NWO [mailto:Alfredo.A.Rios@usace.army.mil]
Sent: Tuesday, June 29, 2010 8:27 AM
To: Melanie D. Chenard
Cc: Johnson, Daniel B NWO
Subject: RE: Survey benchmark issues for shoreline project @ Cherry Creek Reservoir
Melanie, following is the comments I received from the district office;
------Original Message-----
From: Behm, Randall L NWO
Sent: Tuesday, June 29, 2010 6:01 AM
To: Rios, Alfredo A NWO; Letak, Ben J NWO
Cc: Schultz, Lloyd H NWO; Kragt, Jon E NWO; Seefus, Kathryn J NWO; Pletka, Angela NWO
Subject: RE: Survey benchmark issues for shoreline project @ Cherry Creek Reservoir
Fred,
With regards to question 5 , you are correct, the cut and fill balance is not required for the area below the conservation pool. However, work in this area should be documented.

I understand that the survey datum has become an item of discussion for the reservoir. While that concern will be resolved through future survey efforts it should not impact the level of design or review for any specific projects identified to occur along the shoreline. Each project should be designed using one datum only. The design of the excavation material and the fill material should be based upon only one datum. In essence, each project can be viewed as using a local construction datum, if needed. That way we can perform a review of the cut and fill and not have to worry about a datum shift occurring within the project limits. We want to ensure that the area capacity curve is not adversely impacted by the addition of fill material.

Randy
Randall L. Behm, P.E. CFM
Chief, Flood Risk and Floodplain Management Section
Hydrologic Engineering Branch - Omaha District
Office: (402) 995-2322

Fax: (402) 995-2321
e-mail: randall.I.behm@usace.army.mil
Certified Floodplain Manager
I know this doesn't directly answer question 1 and 2, but it gives you direction on 3,4, and 5. I believe the answers to your questions are as
follows:

1. Correct, based on conversations I have had with district staff.
2. Waiting for an answer to this, but shouldn't impact your design.
3. Use datum used in past, based on 1929 NGVD as you have used on past projects. Noting the datum difference and include a conversion on your design drawings in not necessary?
4. The cut/fill balance should be the datum used in past for other projects.
5. A cut/fill balance below the conservation pool is not necessary, but the area impacts should be document.

Is this response sufficient to allow you to move forward with your design?

Fred Rios
Operations Project Manager
Tri-Lakes Project Office
-----Original Message-----
From: Melanie D. Chenard [mailto:mchenard@MULLERENG.COM]
Sent: Thursday, June 24, 2010 3:36 PM
To: Rios, Alfredo A NWO
Subject: Survey benchmark issues for shoreline project @ Cherry Creek Reservoir
Fred,

As you know, we have been looking into a survey issue that has come to light in our work on the Mountain \& Lake Loop Shoreline Stabilization project.
We
have been in contact with various USACE personnel in the Surveys, Mapping \& GIS Section of the Omaha District to help define the problem and would like to inform you of our findings and receive direction on how to proceed on this project.

## Project Overview

We are currently under contract with the Cherry Creek Basin Water Quality Authority to address erosion and degradation along 2300 linear feet of shoreline at Cherry Creek Reservoir near the Mountain Loop and Lake Loop recreation areas. The proposed work will include riprap and boulder protection along the shoreline along with a series of jetty points intended to deflect erosive currents and provide recreational opportunities. In addition, surface runoff quality will be improved through the reconfiguration of an existing parking lot, construction of infiltration areas, and improvement or elimination of social trails.

## Survey Benchmark Issues

A field survey was conducted in October of 2009 to develop project mapping.
The benchmark used for this survey is a local benchmark with a published elevation on the NGVD29 vertical datum, and has been used as the project benchmark for numerous projects within the Cherry Creek Reservoir pool. The
reservoir water surface elevations measured during the field survey were later compared to the published pool levels on the Colorado Surface Water Conditions website, maintained by the Colorado Division of Water Resources.
It was noted that the surveved water surface elevations were approximatelv $0.9^{\prime}$ lower than the published elevations for the dates of the field survey.

Because the vertical component of this project is critical both from a design standpoint in the establishment of anticipated reservoir levels and from a review standpoint in the balance of cut and fill material for the preservation of flood storage, we endeavored to resolve this $0.9^{\prime}$
discrepancy
to ensure that the project design and construction would be based on the same datum used by the Corps for management of the Reservoir.

Additional field survey was conducted to compare the project datum to other local benchmarks in the area, and to two USACE control points near the reservoir. The original project datum was confirmed to match published
NGVD29 elevations for the local benchmarks and to differ from the USACE control points by $0.97^{\prime}$. Correspondence with the USACE Omaha District - Surveys, Mapping \& GIS Section revealed that the Cherry Creek Reservoir "Proiect Elevations" (Reservoir datum) is only loosely based on NGVD29, and that a 1983 USACE survey identified the Reservoir datum to he $0954^{\prime}$
higher
than NGVD29. This difference applies only to select USACE control points (CP
1 -CP 6), but does carry through to the published reservoir water surface elevations. (Selected email correspondence is attached.)

We understand that the USACE administers the reservoir based on a conservation pool elevation of 5550.0. What has come to light through this inquiry is that the 5550.0 elevation is based on the Reservoir datum rather than the NGVD29 as commonly believed. This means that the conservation pool elevation translates to 5549.0 on NGVD29. However, it appears that the reservoir is administered with the assumption that the Reservoir datum is the same as NGVD29. This leaves us needing some clarification on how to proceed with our project. We believe it is preferable to all parties to keep the project survey on NGVD29 rather than shift to the Reservoir datum. What is not clear is whether the datum difference should be acknowledged on the design plans and in the calculation of cut/fill balance.

Summary
Our investigation has yielded information and raised questions as
follows:

1. The USACE Cherry Creek Reservoir vertical datum is 0.954 ' higher than NGVD29.
2. It appears that this datum difference is not commonly known, and as a result the USACE and Cherry Creek State Park manage the reservoir according to NGVD29.
3. To keep the project on a common datum, the project survey will remain on NGVD29. Should we note the datum difference and include a conversion on our design drawings?
4. Should our cutfill balance between the conservation pool and the spillway be based on 5550.0 NGVD29 (5551.0

Reservoir datum) or on 5549.0 NGVD29 (5550.0 Reservoir datum)?
5. As a separate but related item, we would also like to confirm that a cut/fill balance below the conservation pool is not necessary as it does not impact flood storage.

We understand that you will be forwarding this information to the appropriate authorities who can provide us with the clarification and guidance we require to proceed with this important project. We appreciate the time taken by all in helping identify and address this issue. Please feel free to contact me with any questions. We look forward to your response.

Regards,

USACE Online Source NOAA Online Source DWR Online Source

USACE Online Source Hourly Data (Raw vs Ave ??)

http://www.nwd-mr.usace.army.mil/rcc/programs/data/CHCR
http://water.weather.gov/ahps2/hydrograph to xml.php?gage=egdc2\&output=tabular http://www.dwr.state.co.us/Surfacewater/data/detail tabular.aspx?ID=CHRRESCO\&MTYPE=ELEV

| NOAA Online Source Raw vs Hourly Ave ?? |  | DWR Online Source <br> Raw Data |  |
| :---: | :---: | :---: | :---: |
| Observed Data: |  |  |  |
| \|Date(UTC)| | \|Stage| | Date/Time (raw) | ELEV (ft) |
| 1/28/2014 1:00 | 5550.11 | 1/28/2014 1:00 | 5550.11 |
| 1/28/2014 2:00 | 5550.09 | 1/28/2014 2:00 | 5550.10 |
| 1/28/2014 3:00 | 5550.11 | 1/28/2014 3:00 | 5550.12 |
| 1/28/2014 4:00 | 5550.12 | 1/28/2014 4:00 | 5550.12 |
| 1/28/2014 5:00 | 5550.10 | 1/28/2014 5:00 | 5550.10 |
| 1/28/2014 6:00 | 5550.11 | 1/28/2014 6:00 | 5550.11 |
| 1/28/2014 7:00 | 5550.12 | 1/28/2014 7:00 | 5550.12 |
| 1/28/2014 8:00 | 5550.11 | 1/28/2014 8:00 | 5550.10 |
| 1/28/2014 9:00 | 5550.10 | 1/28/2014 9:00 | 5550.10 |
| 1/28/2014 10:00 | 5550.12 | 1/28/2014 10:00 | 5550.12 |
| 1/28/2014 11:00 | 5550.12 | 1/28/2014 11:00 | 5550.12 |
| 1/28/2014 12:00 | 5550.10 | 1/28/2014 12:00 | 5550.10 |
| 1/28/2014 13:00 | 5550.11 | 1/28/2014 13:00 | 5550.11 |
| 1/28/2014 14:00 | 5550.12 | 1/28/2014 14:00 | 5550.12 |
| 1/28/2014 15:00 | 5550.10 | 1/28/2014 15:00 | 5550.11 |
| 1/28/2014 16:00 | 5550.10 | 1/28/2014 16:00 | 5550.10 |
| 1/28/2014 17:00 | 5550.12 | 1/28/2014 17:00 | 5550.12 |
| 1/28/2014 18:00 | 5550.12 | 1/28/2014 18:00 | 5550.12 |
| 1/28/2014 19:00 | 5550.10 | 1/28/2014 19:00 | 5550.11 |
| 1/28/2014 20:00 | 5550.11 | 1/28/2014 20:00 | 5550.11 |
| 1/28/2014 21:00 | 5550.12 | 1/28/2014 21:00 | 5550.12 |
| 1/28/2014 22:00 | 5550.11 | 1/28/2014 22:00 | 5550.12 |
| 1/28/2014 23:00 | 5550.10 | 1/28/2014 23:00 | 5550.12 |
| 1/29/2014 0:00 | 5550.12 | 1/29/2014 0:00 | 5550.12 |

## DWR Online Source

 Hourly Averages| Date/Time (raw) | ELEV (ft) |
| :---: | :---: |
| 1/28/2014 1:00 | 5550.11 |
| 1/28/2014 2:00 | 5550.11 |
| 1/28/2014 3:00 | 5550.11 |
| 1/28/2014 4:00 | 5550.12 |
| 1/28/2014 5:00 | 5550.11 |
| 1/28/2014 6:00 | 5550.11 |
| 1/28/2014 7:00 | 5550.11 |
| 1/28/2014 8:00 | 5550.11 |
| 1/28/2014 9:00 | 5550.11 |
| 1/28/2014 10:00 | 5550.11 |
| 1/28/2014 11:00 | 5550.11 |
| 1/28/2014 12:00 | 5550.11 |
| 1/28/2014 13:00 | 5550.11 |
| 1/28/2014 14:00 | 5550.12 |
| 1/28/2014 15:00 | 5550.11 |
| 1/28/2014 16:00 | 5550.11 |
| 1/28/2014 17:00 | 5550.12 |
| 1/28/2014 18:00 | 5550.11 |
| 1/28/2014 19:00 | 5550.12 |
| 1/28/2014 20:00 | 5550.11 |
| 1/28/2014 21:00 | 5550.12 |
| 1/28/2014 22:00 | 5550.12 |
| 1/28/2014 23:00 | 5550.12 |
| 1/29/2014 0:00 | 5550.12 |


| From: | Wolf, Craig [CWolf@geiconsultants.com](mailto:CWolf@geiconsultants.com) |
| :--- | :--- |
| Sent: | Wednesday, January 29, 2014 11:11 AM |
| To: | Bill Ruzzo |
| Subject: | Emailing: Cherry Creek Reservoir Elevation Sources |
| Attachments: | Cherry Creek Reservoir Elevation Sources.xlsx |

Bill,
I downloaded the available CCR elevation data for 28 Jan14 from the USACE, NOAA, and DWR websites. NONE of the sites report the same values for each hour of the day... The values are very close (e.g., within 0.01 ft ) but I find it ironic that both NOAA and DWR report the values as coming from the USACE, but they do not match. Part of the issue may be related to instantaneous versus hourly averages but the only website that is $100 \%$ clear on that issue is the DWR site. USACE calls their data "Hourly Data" and NOAA has a mix of "Observed" versus "Forecast" stages in their tabular data set.

I will ask whether the USACE has an opinion or insight into the differences but I'm not confident I will get much clarification. I'm sure I'll get the provisional data statement too....

Craig

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Cherry Creek Project - Effective 09 December 2010
Area in Acres
(2009* Survey)

| Elevation | $\mathbf{0}$ | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ | $\mathbf{6}$ | $\mathbf{7}$ | $\mathbf{8}$ | $\mathbf{9}$ |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $\mathbf{5 5 2 0}$ | 0 | 0 | 0 | 0 | 0 | 61 | 120 | 152 | 184 | $\mathbf{2 1 6}$ |
| $\mathbf{5 5 3 0}$ | 263 | 321 | 369 | 413 | 452 | 486 | 515 | 540 | 559 | 574 |
| $\mathbf{5 5 4 0}$ | 587 | 605 | 627 | 649 | 673 | 698 | 725 | 752 | 780 | 810 |
| $\mathbf{5 5 5 0}$ | 840 | 867 | 894 | 920 | 948 | 977 | 1006 | 1036 | 1066 | 1098 |
| $\mathbf{5 5 6 0}$ | 1128 | 1155 | 1182 | 1211 | 1241 | 1273 | 1307 | 1342 | 1379 | 1418 |
| $\mathbf{5 5 7 0}$ | 1455 | 1488 | 1519 | 1553 | 1589 | 1627 | 1667 | 1710 | 1754 | 1801 |
| $\mathbf{5 5 8 0}$ | 1847 | 1890 | 1932 | 1974 | 2017 | 2060 | 2104 | 2149 | 2193 | 2239 |
| $\mathbf{5 5 9 0}$ | 2285 | 2330 | 2375 | 2420 | 2464 | 2508 | 2552 | 2595 | 2638 | 2681 |
| $\mathbf{5 6 0 0}$ | 2723 | 2765 | 2808 | 2851 | 2894 | 2938 | 2982 | 3026 | 3071 | 3116 |
| $\mathbf{5 6 1 0}$ | 3159 | 3201 | 3242 | 3285 | 3330 | 3376 | 3424 | 3473 | 3524 | 3577 |
| $\mathbf{5 6 2 0}$ | 3628 | 3674 | 3720 | 3767 | 3817 | 3870 | 3924 | 3981 | 4041 | 4102 |
| $\mathbf{5 6 3 0}$ | 4164 | 4223 | 4281 | 4338 | 4395 | 4453 | 4510 | 4568 | 4625 | 4682 |
| $\mathbf{5 6 4 0}$ | 4740 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

*2009 Survey is a composite of data collected in 2006, 2007, and 2009.

| Cherry Creek Project - Effective 09 December 2010 Capacity in Acre-Feet (2009* Survey) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Elevation | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
| 5520 | 0 | 0 | 0 | 0 | 0 | 17 | 122 | 258 | 426 | 626 |
| 5530 | 858 | 1153 | 1500 | 1892 | 2326 | 2796 | 3298 | 3827 | 4378 | 4946 |
| 5540 | 5526 | 6121 | 6737 | 7375 | 8036 | 8721 | 9433 | 10171 | 10937 | 11732 |
| 5550 | 12558 | 13412 | 14293 | 15200 | 16134 | 17097 | 18089 | 19110 | 20161 | 21243 |
| 5560 | 22357 | 23499 | 24668 | 25864 | 27090 | 28347 | 29637 | 30961 | 32321 | 33719 |
| 5570 | 35157 | 36630 | 38133 | 39669 | 41240 | 42848 | 44495 | 46183 | 47915 | 49692 |
| 5580 | 51517 | 53387 | 55298 | 57252 | 59247 | 61286 | 63368 | 65495 | 67666 | 69882 |
| 5590 | 72144 | 74452 | 76804 | 79202 | 81645 | 84131 | 86661 | 89235 | 91852 | 94512 |
| 5600 | 97214 | 99958 | 102745 | 105575 | 108447 | 111364 | 114324 | 117328 | 120376 | 123470 |
| 5610 | 126608 | 129789 | 133010 | 136274 | 139581 | 142934 | 146334 | 149782 | 153281 | 156831 |
| 5620 | 160435 | 164087 | 167784 | 171527 | 175319 | 179162 | 183059 | 187011 | 191022 | 195093 |
| 5630 | 199227 | 203422 | 207674 | 211984 | 216350 | 220775 | 225256 | 229796 | 234392 | 239046 |
| 5640 | 243757 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

[^4]In areas where LiDAR has not been collected, it may be more cost effective to send a survey crew up to survey the sediment range lines instead of flying LiDAR. Using the historic method of the average end area to calculate storage capacity curves would have to be done in those situations. However, if there are multiple studies ongoing at the project, LiDAR's capability to acquire ground elevations at a much denser rate may be more beneficial to a greater number of studies and perhaps then a more cost effective method.

Comparing the two methods of InRoads and Average End Area to calculate storage capacity, seems to give us higher capacity in the reservoir especially at lower lake levels. Any time both LiDAR and current storage capacity tables are available, additional comparison of the two methods would aid in a better understanding or further support or oppose the finding for Cherry Creek.

Table G-1. Area Capacity Calculated using the Alternative LiDAR-based InRoads Calculation Method

| Cherry Creek Reservoir - LiDAR Based Alternative Calculation Method Area Capacity in Acre-Feet ( 2009-2010 Survey) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| NAVD 88 Elevation | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
| 5520 | 0 |  | 0 |  | 1 |  | 34 | 220 | 405 | 688 |
| 5530 | 971 | 1320 | 1668 | 2071 | 2474 | 2920 | 3366 | 3863 | 4359 | 4902 |
| 5540 | 5445 | 6039 | 6632 | 7269 | 7905 | 8589 | 9273 | 10018 | 10763 | 11575 |
| 5550 | 12387 | 13242 | 14097 | 14992 | 15886 | 16841 | 17795 | 18812 | 19828 | 20909 |
| 5560 | 21989 | 23138 | 24287 | 25498 | 26709 | 27984 | 29259 | 30608 | 31956 | 33371 |
| 5570 | 34786 | 36264 | 37741 | 39283 | 40825 | 42436 | 44046 | 45736 | 47426 | 49196 |
| 5580 | 50966 | 52818 | 54670 | 56611 | 58551 | 60576 | 62600 | 64711 | 66822 | 69020 |
| 5590 | 71218 | 73501 | 75783 | - 78147 | 80511 | 82962 | 85412 | 87951 | 90490 | 93114 |
| 5600 | 95738 | 98444 | 101149 | 103938 | 106727 | 109607 | 112486 | 115462 | 118437 | 121508 |
| 5610 | 124579 | 127741 | 130902 | 134147 | 137391 | 140721 | 144051 | 147473 | 150895 | 154415 |
| 5620 | 157935 | 161557 | 165179 | 168907 | 172635 | 176469 | 180302 | 184240 | 188177 | 192224 |
| 5630 | 196271 | 200429 | 204587 | 208861 | 213134 | 217537 | 221939 | 226471 | 231003 | 235662 |
| 5640 | 240321 | 245110 | 249899 | 254822 | 259744 | 264799 | 269853 | 275038 | 280222 | 285542 |
| 5650 | 290862 |  |  |  |  |  |  |  |  |  |

*Blue numbers are estimated using the average of the bordering two values.

[^5]


[^0]:    ${ }^{1}$ Agreement dated December 5, 2013
    ${ }^{2}$ Agreement dated November 13, 2013.

[^1]:    ${ }^{3}$ Email from Wes Friesen (ANR) dated 1/28/2014 Subject: "Cherry Creek Elevations" (see appendix to this memo).
    ${ }^{4}$ Email from Craig Wolf (GEI) dated January 30, 2014 Subject: "Action items from conference call" (see appendix to this memo).
    ${ }^{5}$ USACE July 2011. Tri-Lakes Sedimentation Studies Area-Capacity Report. Page 2-7. (see appendix to this memo)
    ${ }^{6}$ Email from Melanie Chenard (Muller Engineering) dated June 30, 2010. "RE: Survey benchmark issues for shoreline project @ Chery Creek Reservoir".
    ${ }^{7}$ Elevation data were downloaded from the NOAA, USACE, and the Colorado Division of Water Resources websites by GEI, compared, and found to be within 0.01 -feet. (see appendix to this memo for comparison table and website links)

[^2]:    ${ }^{8}$ In USACE 2011, they report normal pool to be from 5504 to 5550 . The difference with the ANR survey could be sediment accumulation and/or precision of side-scan radar.
    ${ }^{9}$ USACE 2011. p. D-8, (see appendix to this memo)
    ${ }^{10}$ It is interesting to note that if the elevations for the USACE data are shifted by only 1-foot, the curves for the ANR and USACE storage volume are graphically aligned suggesting that vertical datum differences may account for the volume differences.

[^3]:    ${ }^{11}$ (4-years x $12-\mathrm{af} / \mathrm{yr}=48$-af; $48 / 964=0.05$ )
    ${ }^{12}$ USACE 2011. p. G-3. (see appendix to this memo)
    ${ }^{13}$ USACE 2011. p. F-7. (see appendix to this memo)

[^4]:    *2009 Survey is a composite of data collected in 2006, 2007, and 2009.

[^5]:    To compare the storage capacity for elevation 5550 based on Local Project Datum, use elevation 5551.8 storage value based NAVD88 datum.

    NAVD88: Volume at $5551=13,242$; Volume at $5552=14097$; then Volume at $5551.8=13,926$ af

